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Name: [Redacted]  
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Section: [Redacted]

**Exam II**

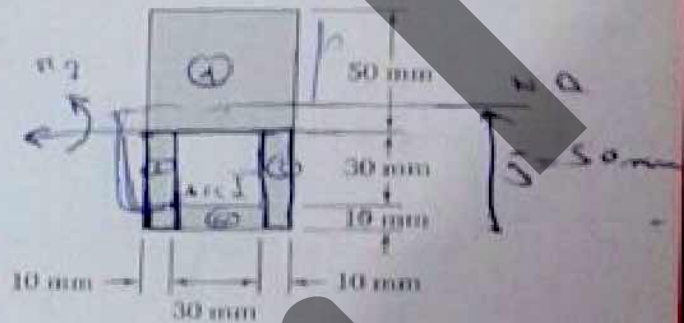
Friday May 16, 2014

Duration: 1h 30min

All work must be shown to receive full credit.

**Problem 1.** (20 pts.) A beam of the cross section shown is extruded from an aluminum alloy. If the moment applied about the z axis (horizontal axis directed to the left) is  $M_z = 10 \text{ kN.m}$ , calculate:

- a - The maximum tensile stress. (12 pts.)
- b - The maximum compressive stress. (4 pts.)
- c - The stress at point A. (4 pts.)



a)  $\sigma_{\text{max}} = \frac{M \cdot c}{I}$

$c = \frac{\text{Rectangle Height} - \text{Hole Height}}{2}$

$c = \frac{45 \times 30 \times 50 - 25 \times 30 \times 30}{30 \times 50 - 30 \times 30}$

$c = \frac{202500 - 22500}{4500 - 900} = 50 \text{ mm}$

$I = I_1 + 2I_2 + I_3$

$I_1 = \frac{1}{12} b h^3 + A d^2 = \frac{1}{12} \times 50 \times 10^3 + 50 \times 50 \times 10^2 \times (15 \times 10^{-3})^2$   
 $= 5,208 \times 10^7 + 5,625 \times 10^7 = 1,0833 \times 10^8 \text{ m}^4$

$I_2 = 2 \left[ \frac{1}{12} b h^3 + A d^2 \right] = 2 \left[ \frac{1}{12} \times 1 \times 10^3 \times (60 \times 10^{-3})^3 + 40 \times 10 \times 10^2 \times (30 \times 10^{-3})^2 \right]$   
 $= 2 \left[ 3,33 \times 10^7 + 3,6 \times 10^7 \right]$

$= 2 \left[ 3,33 \times 10^7 + 3,6 \times 10^7 \right]$

$= 3,266 \times 10^8 \text{ m}^4$

$$I_a = \frac{1}{12} b h^3 + A d^2 = \frac{1}{12} 30 \times 10^3 \times (40 \times 10^{-3})^3 + 30 \times 10 \times 10^3 \times (45 \times 10^{-3})^2$$

$$= 2,5 \times 10^{-6} + 6,075 \times 10^{-6}$$

$$= 8,575 \times 10^{-6} \text{ m}^4$$

$$I = I_1 + I_2 + I_3$$

$$I = 2,52 \times 10^{-6} \text{ m}^4$$

a) tension occurs at bottom:

$$\sigma = - \frac{M c_{\text{bot}}}{I} = - \frac{10 \times 10^3 \times (-50 \times 10^{-3})}{2,52 \times 10^{-6}} = 198,412 \times 10^6 \text{ Pa}$$

$$= 198,412 \text{ MPa}$$

$$\sigma_{\text{tension}} = 198,412 \text{ MPa}$$

↳ compression at top:

$$\sigma_{\text{top}} = - \frac{M c_{\text{top}}}{I} = - \frac{10 \times 10^3 \times 40 \times 10^{-3}}{2,52 \times 10^{-6}} = -158,73 \text{ MPa}$$

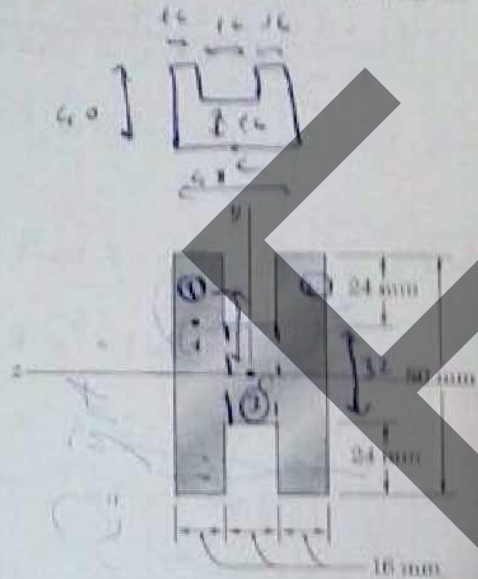
$$\sigma_{\text{top}} = 158,73 \text{ MPa}$$

$$\sigma_{\text{top}} = 158,73 \text{ MPa}$$

$$c) \sigma_a = - \frac{M c_a}{I} = - \frac{10 \times 10^3 \times (-40 \times 10^{-3})}{2,52 \times 10^{-6}} = 158,73 \text{ MPa}$$

Problem 2: (20 pts.) The cross section shown is subjected to a shear force of 10 kN directed along the y axis. Calculate:

- a - The maximum shear stress at A. (12 pts.)
- b - The shear stress at C. (8 pts.)



due to symmetry

$$\bar{y} = 40 \text{ mm}$$

$$I = I_1 + I_2 + I_3$$

$$I_1 = I_2 = \frac{1}{12} \times (16 \times 10^{-3}) \times (80 \times 10^{-3})^3$$

$$= 6.8267 \times 10^{-8} \text{ m}^4$$

$$I_3 = \frac{1}{12} b h^3 = \frac{1}{12} \times (16 \times 10^{-3}) \times (32 \times 10^{-3})^3$$

$$= 4.37 \times 10^{-8} \text{ m}^4$$

$$I = 2 I_1 + I_3 = 2 \times (6.8267 \times 10^{-8}) + 4.37 \times 10^{-8}$$

$$I = 1.40904 \times 10^{-6} \text{ m}^4$$

a) since  $A_1$  located above NA, find  $A_{11}$  above point A  $\times 2$

$$Q_A = 2 \times A \times \bar{y}$$

$$= 2 \times 16 \times 24 \times 10^{-6} \times 28 \times 10^{-3}$$

$$= 2.1504 \times 10^{-6} \text{ m}^3$$

$$Q_A = 2.1504 \times 10^{-6} \text{ m}^3$$

$$Q_A = 16 \times 10^{-6} \times 2 = 0.032 \text{ m}$$

$$\sigma_A = \frac{V Q_A}{I t_A} = \frac{10 \times 10^3 \times 1.2298 \times 10^5}{0.32 \times 4.40304 \times 10^6}$$

$$\sigma_A = 2.7255 \text{ MPa}$$

$$= \frac{10 \times 10^3 \times 2.1504 \times 10^5}{0.32 \times 4.40304 \times 10^6} = 4.77 \text{ MPa}$$

bt find  $Q$  for Area above  $C$  for max value

$$Q_C = A_1 \bar{y}_1 + 2A_2 \bar{y}_2$$

$$= 48 \times 16 \times 10^6 \times 8 \times 10^{-3} + Q_A$$

$$= 6.144 \times 10^6 + 2.1504 \times 10^5$$

$$= 2.7648 \times 10^5 \text{ m}^3$$

$$t = 48 \times 10^{-3} \text{ m}$$

$$\sigma_C = \frac{V Q_C}{I t_C} = \frac{10 \times 10^3 \times 2.7648 \times 10^5}{48 \times 10^{-3} \times 4.40304 \times 10^6}$$

$$= 4.088 \text{ MPa}$$

Problem 3. (20 pts.) The beam shown below has a rectangular cross-sectional area of  $0.2 \times 0.4 \text{ m}^2$ .

- a - Draw the shear and bending moment diagrams. (14 pts.)
- b - Calculate the maximum shear stress in the beam. (6 pts.)

a)  $\sum \pi_c = 0 \Rightarrow$

$$-12 \times 5.2 + R_B \times 4 - 16 \times 4 \times 2 = 0$$

$$R_B = 47.6 \text{ kN}$$

$\sum F = 0;$

$$-12 + 47.6 - 16 \times 4 + R_C = 0$$

$$R_C = 29.4 \text{ kN}$$



$$V_A = -12 \text{ kN}$$

$$V_B^- = V_A - \int_{1.2}^4 16 dx = V_A$$

$$V_B^+ = V_A + R_B = 35.6 \text{ kN}$$

$$V_C = V_B^+ - \int_{4}^{8} 16 dx = 35.6 - 16 \times 4 = -29.4 \text{ kN}$$

$$M_A = 0$$

$$\frac{M}{35.6} = \frac{4-m}{29.4}$$

$$29.4 M = 35.6 (4-m)$$

$$29.4 M = 142.4 - 35.6 m$$

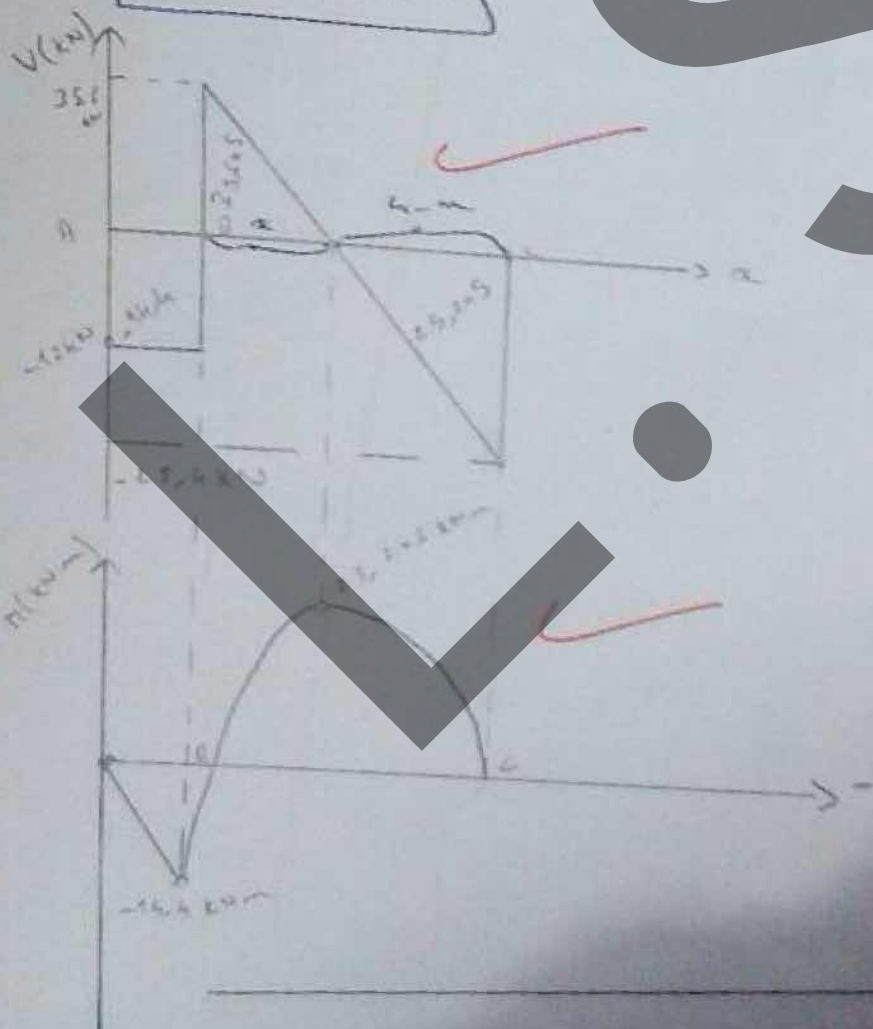
$$64 M = 142.4$$

$$M = 2.225 \text{ m}$$

$$R_B = -16.6 \text{ kN}$$

$$M_{max} = -16.6 + 35.6 \times 2.225 = 11.8 \text{ kNm}$$

$$M_C = 0$$



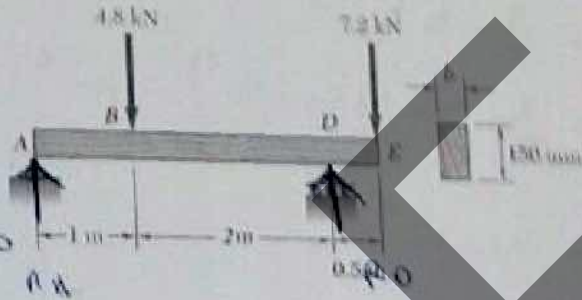
$$b) \quad v_{\text{max}} = |35.4 \text{ km}| \quad \text{at } B$$

•  $\frac{d}{dx}$  for rectangular cross section

$$v_{\text{max}} = \frac{3}{2} \frac{v}{A} = \frac{3}{2} \times \frac{35.4 \times 10^3}{9.2 \times 10^4}$$

$$= 667500 \text{ Pa}$$

Problem 4. (20 pts.) For the beam and loading shown, determine the minimum required width  $b$ , knowing that for the grade of timber used,  $\sigma_{all} = 14 \text{ MPa}$  and  $\tau_{all} = 900 \text{ kPa}$ .



$$\sum \tau_D = 0$$

$$R_A \times 3 - 4.8 \times 2 + 7.2 \times 0.5 = 0$$

$$R_A = 2 \text{ kN}$$

$$\tau_A = \tau_D = 0$$

$$\sum F_C = 0$$

$$R_A - 4.8 - 7.2 + R_D = 0$$

$$R_D = 10 \text{ kN}$$

$$V_A = 2 \text{ kN}$$

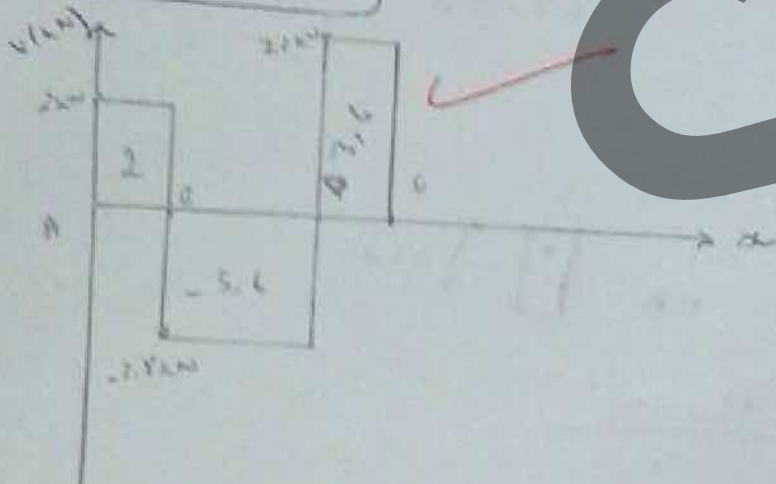
$$V_B = 2 \text{ kN}$$

$$V_D^+ = 2 - 4.8 = -2.8 \text{ kN}$$

$$V_D^- = -5.1 \text{ kN}$$

$$V_E^- = -5.1 + 7.2 = 2.1 \text{ kN}$$

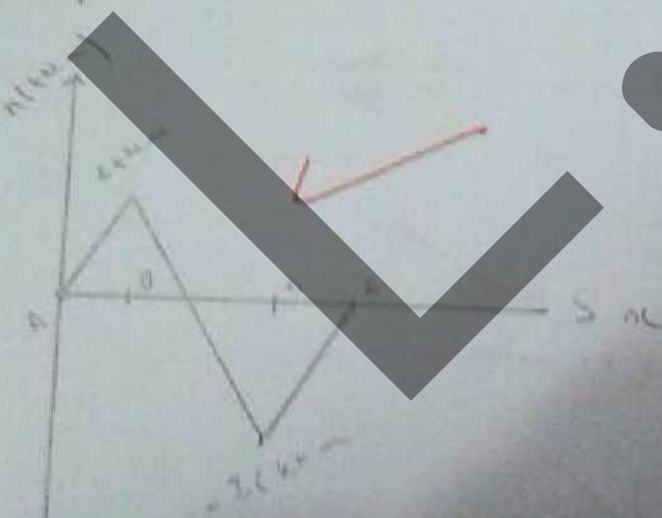
$$V_C = 7.2 - 7.2 = 0$$



$$\tau_B = 2 \text{ kNm}$$

$$M_D = 2 - 2.0 = -2 \text{ kNm}$$

$$R_C = -1.6 + 2.1 = 0$$



$$C_{max} = |2 \text{ kN}|$$

$$M_{max} = |2.1 \text{ kNm}| \text{ at } D$$

negative  $\Rightarrow$  tension at top  
compression at bottom

$$\sigma = \frac{M c}{I}$$

$$I = \frac{1}{12} b h^3$$

$$= \frac{1}{12} b \times (150 \times 10^{-3})^3 \text{ m}^4$$

$$c = 75 \times 10^{-3} \text{ m}$$

$$14 \times 10^6 = \frac{3,6 \times 10^3 \times 75 \times 10^{-3}}{\frac{1}{12} b (150 \times 10^{-3})^3}$$

$$b = \frac{12 \times 3,6 \times 10^3 \times 75 \times 10^{-3}}{14 \times 10^6 \times (150 \times 10^{-3})^3} = \frac{3240}{47250} = 68,57 \text{ mm}$$

$$\tau = \frac{V Q}{I t}$$



$$Q_{\text{max}} = \frac{150 \times 10^{-3}}{2} \times b \times \frac{150 \times 10^{-3}}{4}$$

rectangular cross section  $\Rightarrow C_{\text{max}} = \frac{3}{8} \frac{V}{A}$

$$300 \times 10^3 = \frac{3 \times 7,2 \times 10^3}{8 \times 150 \times 10^{-3} \times b}$$

$$b = \frac{21,6 \times 10^3}{300 \times 10^3 \times 900 \times 10^{-6}} = 80 \text{ mm}$$

$$b = 80 \text{ mm}$$



Problem 5. (20 pts.) Determine the largest permissible distributed load  $w$  for the beam and loading shown, knowing that the allowable normal stress in tension is  $\sigma_{t,all} = 160 \text{ MPa}$  and in compression  $\sigma_{c,all} = -130 \text{ MPa}$ .



$$\bar{y} = \frac{\bar{y}_1 A_1 + \bar{y}_2 A_2}{A_1 + A_2} = \frac{70 \times 60 \times 20 + 30 \times 20 \times 60}{60 \times 20 + 60 \times 20} = 50 \text{ mm}$$

$$I = I_1 + I_2$$

$$I_1 = \frac{1}{12} \times 60 \times 10^{-3} \times (60 \times 10^{-3})^3 + 60 \times 10^{-3} \times (20 \times 10^{-3})^2$$

$$I_1 = 6 \times 10^{-8} + 4.8 \times 10^{-7} = 5.2 \times 10^{-7} \text{ m}^4$$

$$I_2 = \frac{1}{12} \times 20 \times 10^{-3} \times (60 \times 10^{-3})^3 + 60 \times 10^{-3} \times (20 \times 10^{-3})^2$$

$$I_2 = 3.6 \times 10^{-7} + 6.8 \times 10^{-7} = 8.4 \times 10^{-7} \text{ m}^4$$

$$I = I_1 + I_2 = 5.2 \times 10^{-7} + 8.4 \times 10^{-7} = 1.36 \times 10^{-6} \text{ m}^4$$

$$\sum \mathcal{M}_C = 0;$$

$$R_A \times 2 - u \times 4 \times 0,5 + u \times 2 \times 1 = 0$$

$$2 R_A = -1,5 u$$

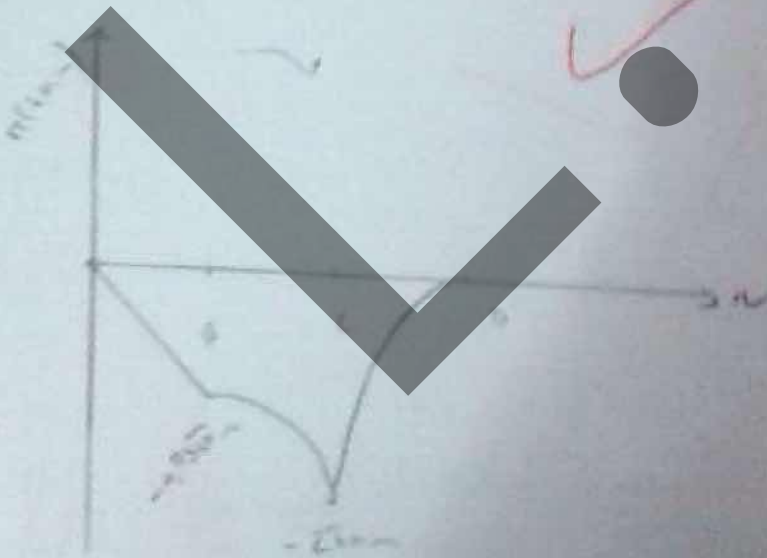
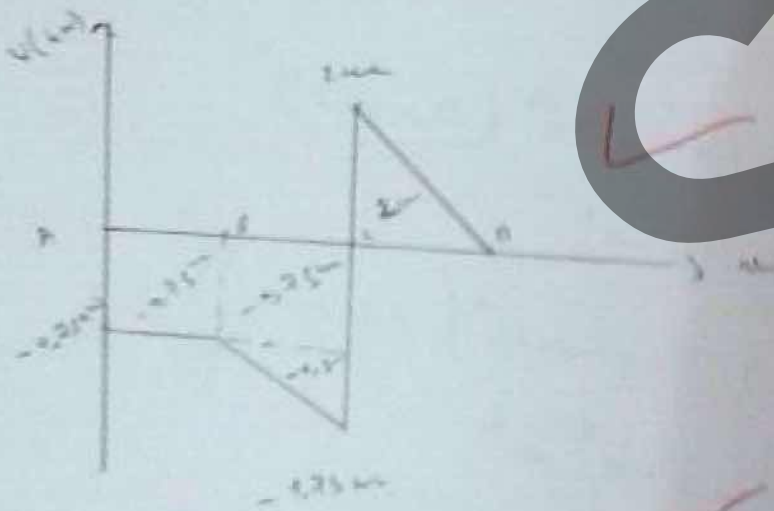
$$R_A = -0,75 u \quad (\downarrow)$$

$$\sum \mathcal{F}_x = 0$$

$$-R_A - u \times 3 + R_C = 0$$

$$-0,75 u - 3u = -R_C$$

$$R_C = 3,75 u$$



$$\tau = |0.75 \text{ cm}|$$

$$\sigma_T = \frac{\tau C_{top}}{I}$$

$$M = \frac{\sigma_T I}{C_{top}} = \frac{160 \times 10^6 \times 1.3 \times 10^{-6}}{20 \times 10^{-3}} = 0.75 \text{ cm}$$

$$W_1 = 14506.6 \text{ N}$$

$$\sigma_c = \frac{\tau C_{bottom}}{I} = \underline{0.75 \text{ cm}}$$

$$M = \frac{\sigma_c I}{C} = 0.75 \text{ cm} = \frac{-130 \times 10^6 \times 1.3 \times 10^{-6}}{-50 \times 10^{-3}}$$

$$W_2 = 4714.667 \text{ N}$$

$$\tau = |2 \text{ cm}|$$

$$\sigma_T = \frac{\tau C}{I} = 2 \text{ cm}$$

$$W_3 = \frac{\sigma_T I}{C \times 2} = 5440 \text{ N}$$

$$W_4 = \frac{\sigma_c I}{C_{bottom} - x} = 1765 \text{ N}$$

$$W_4 = 1765 \text{ N/m}$$

answer